

# HYDRAULICS

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Chapt 1

1. Def

- Fluid: A substance that deforms continuously under a shear force  
(in liquid or gas phase)

- Shear stress applied to solid  $\rightarrow$   
 $\rightarrow$  deform or bend to a certain level/  
liquid gas

not easily compressed  
comple compressed  
occupy fixed volume

2. Unit

BG sys:

- ① length: foot (ft) ② time: s
- ③ mass: slg ④ force: pound (lb)
- ⑤ temperature: ~~F~~ Fahrenheit ( $^{\circ}$ F)  $^{\circ}F$   
absolute temp in Rankine ( $^{\circ}$ R)  $^{\circ}R = ^{\circ}F + 459.67$
- ⑥  $^{\circ}C = (^{\circ}F - 32) \frac{5}{9}$

EE sys:

- ① length: foot (ft) ② time: s
- ③ temp: abs temp in Rankine ( $^{\circ}$ R)
- ④ mass: pound mass (lbm)
- ⑤ pound (lb)



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## 6. Flow visualization method

- 1) path lines

line tangent to direction of velocity at every point in the flow at  $t$

$$\text{S.L. expression: } \frac{dy}{dx} = \frac{v}{u}$$

Expt. velocity field given by  $\vec{V} = 0.3x^2 - 0.3y \hat{i}$   
 ① velocity of particle at  $(2, 8)$

② equation of streamlines

plot streamlines 9) stress field passing through (2, 8)

if the particle passing through  $(x_0, y_0)$  is

located at  $t=0$ ,

determine location of it at  $t=s$ ,

show the equation of the particle path is

the same as equation of stream line.

$$1) \vec{V}(x, y) = 0.6\hat{i} - 2.4\hat{j}$$

$$\Rightarrow \frac{dy}{dx} = \frac{-0.3y}{0.3x} = -\frac{y}{x}$$

$$\therefore \frac{dy}{x} = -\frac{y}{x} dt$$

$$3) xy = C$$

$$\text{Infinite no. of streamlines}$$

$$\text{but only one passes (2, 8)}$$

$$4) u = \frac{dx}{dt} = 0.3x$$

$$v = \frac{dy}{dt} = -0.3y$$

$$x = x_0 e^{0.3t}$$

$$y = y_0 e^{-0.3t} \Rightarrow t=6, (x_0, y_0)$$

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## 8. Viscosity (internal friction.) Date No

measured of fluid in relative to how

inherent tension.

Thinner.

Fluid element

at time  $t+dt$

velocity  $V$

$u=0$

fluid element at time  $t+dt$

$\delta x$

$\delta y$

$\delta z$

$\delta L$

$\delta A$

$\delta V$

$\delta F_x$

$\delta F_y$

$\delta F_z$

$\delta F$

$\delta T$

$\delta \tau$

$\delta \sigma$

$\delta \mu$

$\delta \eta$

$\delta \nu$

$\delta \rho$

(Q5-2): types of fluids

exp following using  $F_2/T$ : Newtonian fluids

- Pressure sys - shear stress is proportional to deformation rate:  $\tau = k \frac{\dot{\epsilon}}{T}$
- mass flux
- Volume flow rate - most of common fluids: e.g. water, oil, gasoline
- $P = F T^2 / L^4$
- $P_{visc} = F L^2$
- Non-Newtonian fluids - shear stress not proportional to deformation rate
- energy =  $F L$
- $m_{flux} = F T / L^3$
- $V_{flow,real} = L^3 / T$

- bingham fluids shear thinning  
and shear thickening fluids ...

10. Surface tension

1) Develops at interfaces with other liquids  
at  $20^\circ\text{C}$ ,  $(1 \text{ dyne/cm}^2)$  or solids due to the attraction between molecules

2) Surface tension = force / length ( $\text{N/m}$ )

Wet  $\frac{\text{water}}{\text{surface}} \sim 0.0728 \text{ N/m}$

Wet  $\frac{\text{mercury}}{\text{surface}} \sim 0.023 \text{ N/m}$

3) Interface acts like a stretched elastic membrane (with a contact angle)

4) Creating surface tension  $\sigma$  depends on the type of liquid and contact surface

Example of Non-Newtonian fluid = Oobleck = cornstarch water

**10.1 Pressure inside a drop of fluid**

O: surface tension ( $N/m$ )

$$\Delta p = \rho g - \sigma$$

( $\rho$ : internal pressure,  $\sigma$ : external pressure)

force around the edge:  $2\pi R \sigma$

force due to pressure difference:  $\Delta p \cdot \pi R^2$

balance:  $2\pi R \sigma = \Delta p \cdot \pi R^2$

$$\Rightarrow \Delta p = \frac{4\sigma}{R}$$

**10.2 Eng application**

- insignificant for large scale motions
- important for some small scale motions:
  - e.g. capillary rise (or depression)

**10.3 Bernoulli's principle**

flow in pipe

flow in horizontal pipe

pipe flow

typically horizontal

charge  $\gg$  very low charge


 A   $\rightarrow$   
 non-wetting surface  $\downarrow \pi R^2 h$   
 wet surface  $\downarrow$   
 balance: flat face  $\Rightarrow$  weight  $\Downarrow$   $2\pi R C_D g = \pi R^2 h$   
 $\Downarrow h = \frac{2C_D g}{\gamma R}$ ,  $h \propto \frac{1}{R}$

define uniform pressure +  $\rightarrow$   $p_{atm}$   $\rightarrow$   $p_{atm}$   $\rightarrow$   $p_{atm}$   
 neglect vertical variation across the cross-section  
 free jets  $\rightarrow$   $p_{atm}$   $\rightarrow$   $p_{atm}$   $\rightarrow$   $p_{atm}$   
 fluid jets open to atm all sides  
 e.g. garden hose

compressibility of fluids,  
 1) in compressible fluids - density change negligible:  $\frac{dp}{dt} = 0$   
 liquids & gases with small Mach number ( $M = \frac{V}{c} < 0.3$ ,  $c$ : speed of sound)  
 $V$ : flow velocity

10.1 Pressure inside drop of fluid

$\sigma$ : surface tension ( $N/m$ )

$$\Delta p = \rho g \sigma$$

( $\rho$ : internal pressure,  $\rho_e$ : external pressure)

force around the edge:  $2\pi R \sigma$

force due to pressure difference:  $\Delta p \pi R^2$

balance:  $2\pi R \sigma = \Delta p \pi R^2$

$$\Rightarrow \Delta p = \frac{2\sigma}{R}$$


10.2 Egg approach

- insignificant for large scale motions
- important for some small scale motions:
  - e.g. capillary rise (or depression)

10.3 Pipe flow

- a) Law of hydrostatic pressure. (no motion)
- b) Pipe flow (only in horizontal) typically, horizontal
- c) Bernoulli's principle (vertical motion)


 A   $\rightarrow$   
 non-wetting surface  $\downarrow \pi R^2 h$   
 wet surface  $\rightarrow$   
 balance: flat face  $\Rightarrow$  weight  
 $\hookrightarrow 2\pi R C_D g = \gamma \pi R^2 h$   
 $\hookrightarrow h = \frac{2C_D g}{\gamma R}$ ,  $h \propto \frac{1}{R}$

define uniform pressure +  
 relative vertical variation  
 across the cross-section

b) Free jets  
 $\rightarrow$  fluid jets open  
 (to & from all sides)  
 e.g. garden hose

$\rightarrow P = \rho v^2 / 2$  Bern.

i) Compressibility of fluids.  
 1) in compressible fluids  
 - density change negligible:  $\frac{d\rho}{dt} = 0$   
 - liquids & gases with small Mach number ( $M = V/c < 0.3$ ,  $c$ : speed of sound)

v: flow velocity

2) compressible fluids

- density change **not negligible**
- e.g. flow of gases ( $M \approx 0.3$ ) ; some flow of liquids under extremely high pressure ~~friction~~ (water hammer)

2). Bulk modulus

Bulk compressibility modulus (or modulus elasticity)  
- ratio of pressure change to relative density change (compressibility)

$$- \text{Unit: N/m}^2, \text{ Pascal} \\ - E_v = \frac{dp}{\Delta p/p}$$

3. compression & expansion of gases

- When gases compressed (expanded), relationship between pressure and density depends on the nature of the process

1) under constant temperature condition ( $T = \text{const}$ ):

$$\frac{p}{\rho} = \text{const}, \quad E_v = \rho$$

2) under isentropic conditions (frictionless compression and no heat exchange with surroundings)

$$\frac{p}{\rho^k} = \text{const}, \quad E_v = \rho^k$$

$\$ k$ : ratio of the specific heat at constant pressure,  $c_p$ , to the specific heat at constant volume,  $c_v$  ( $k = c_p/c_v$ )  
(Note: pressures are abs pressures)

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14. speed of sound

- 1) disturbances in fluids travel at speed of sound.

- 2) speed of sound in fluids:

$$c = \sqrt{\frac{\partial P}{\partial \rho}} = \sqrt{\frac{E_v}{\rho}}$$

- 3) for gives undergoing isentropic process  $E_v = k\rho$

$$\text{ideal: } c = \sqrt{kRT}$$

15. Vapour pressure

- pressure developed in a vacuum space left above the liquid in a closed container

- $T \uparrow, p_v \uparrow$

- boiling : when the absolute pressure reaches  $p_v$

- importance in engineering appli:

- for fluid passing Valve and pumps

- when  $P < p_v \Rightarrow$  vapor bubble ('low')

- ⇒ swept along high  $\rightarrow$  bubble collapse

- ⇒ may damage the structures

— cavitation

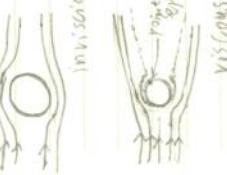
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## 16. Classification of fluid motions

### 16.1 Viscous and inviscid flows

- all real fluids are viscous
- flows assumed inviscid when
  - viscosity not important
  - to tell drag on an airfoil!



### 16.2 Laminar & turbulent flows

- boundary layer
- 1) Laminar flow
    - fluid particles move in smooth layers
  - 2) turbulent flow
    - fluid particles mix rapidly with random three-dimensional velocity fluctuations.

$$\textcircled{1} \quad \vec{V} = U \hat{i} \rightarrow \text{laminar}$$

$$\textcircled{2} \quad \vec{V} = (\bar{u} + u') \hat{i} + v' \hat{j} + w' \hat{k} \rightarrow \text{turbulent}$$

### 16.3 Internal & external flows

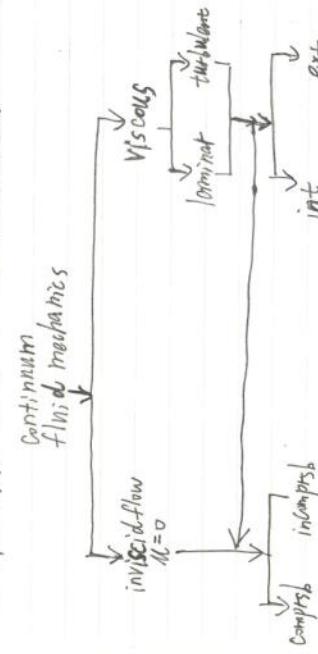
- 1) internal flows
  - flows completely bounded by solid surfaces. (e.g. flow in pipes)
- 2) external flows
  - flows over bodies immersed in unbounded fluid (e.g. flow around a submarine)

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both int & ext flows may be laminar/turbulent  
compressible/in compressible

### Classification of fluid motion



continuum fluid mechanics

Classification of fluid motion

laminar/turbulent

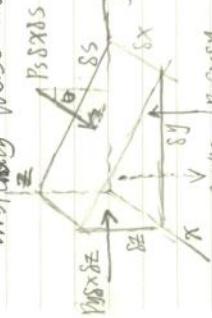
int ext

laminar/turbulent

### Chpt 3 Hydrostatics

Fluid statics {  
 no relative motion between fluid particles ( $\frac{\partial u}{\partial x} = 0$ )  
 no shear stress ( $\Sigma M \frac{\partial u}{\partial y} = 0$ )  
 normal stress (pressure) exists

1. Pascal's law  
 arbitrary wedge-shaped element:



Newton's 2nd law

$$\sum F_y = m a_y : p_s \delta x \delta z - p_s \delta x \delta z \sin \theta = p \frac{\delta x \delta y \delta z}{2} \cdot \gamma y$$

$$\sum z = m a_z : p_z \delta x \delta y - \gamma \frac{\delta x \delta y \delta z}{2} - p_s \delta x \delta z \cos \theta = p \frac{\delta x \delta y \delta z}{2} \cdot \gamma z$$

Since  $\delta y = \delta s \sin \theta$ ;  $\delta z = \delta s \cos \theta$

$$\Rightarrow p_y - p_s = \gamma a_y \frac{\delta y}{2}$$

$$p_z - p_s = (p a_z + \gamma) \frac{\delta z}{2}$$

when  $\delta y \rightarrow 0$  and  $\delta z \rightarrow 0 \Rightarrow p_s = p_y = p_z$

$\Rightarrow$  Pressure at a point is independent of direction (Pascal's law) -  $\Delta$  scalar property of fluid.

2. Pressure variation in a static fluid,  
 1) body force  
 2) surface forces.

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Total forces:

$$X: \sum F_x = -\frac{\partial P}{\partial x} \delta x \delta y \delta z$$

$$Y: \sum F_y = -\frac{\partial P}{\partial y} \delta x \delta y \delta z$$

$$Z: \sum F_z = -\frac{\partial P}{\partial z} \delta x \delta y \delta z - mg$$

Applying Newton's 2nd law

$$\begin{cases} \sum F_x = m a_x = 0 \\ \sum F_y = m a_y \\ \sum F_z = m a_z \end{cases} \quad \begin{cases} \text{since } m = \rho \delta x \delta y \delta z \\ \frac{\partial P}{\partial x} \delta x = m a_x \\ \frac{\partial P}{\partial y} \delta y = -m a_y \\ \frac{\partial P}{\partial z} \delta z = -\rho g = -m a_z \end{cases}$$

$$\begin{cases} a_x = a_y = a_z = 0 \\ \frac{\partial P}{\partial x} = 0 \\ \frac{\partial P}{\partial y} = 0 \\ \frac{\partial P}{\partial z} = 0 \end{cases} \quad \begin{cases} \text{Restrictions:} \\ 1) \text{static fluid} \\ 2) \text{gravity as only body force} \\ 3) z-axis vertical and upward. \end{cases}$$

$$\Rightarrow \frac{\partial P}{\partial z} = -\rho g = -y$$

2.1 Pressure change - Integral form:  
 for incompressible fluids, integrate  $\frac{\partial P}{\partial z} = -\rho g = -y$

$$\int dp = -\gamma \int dz \Rightarrow p = -\gamma z + \text{const}$$

$$\Rightarrow p + \gamma z = \text{const}$$

$$\text{or } \underbrace{p + \gamma z}_{\text{Piezometric head}} = \text{const}$$

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At two points 1, 2 in liquid:

$$\frac{P_1}{\gamma} + z_1 = \frac{P_2}{\gamma} + z_2 \quad \text{or} \quad P_2 - P_1 = \gamma(z_1 - z_2) = \Delta P$$

Examp: At free surface  $z_1 = 0$  (gage)

$$\text{Or } z_1 = 0, P_1 = 0 \rightarrow \text{const} = 0 \\ P_2/\gamma - h = 0 \Rightarrow P_2 = \gamma h = \gamma B h \quad (\text{gage})$$

2.2 Absolute pressure & gage pressure

$P_{abs} = P_{gage} + P_{atm}$   
— Pressure values must be stated with respect to a reference

2.3 Pressure transmission throughout a stationary fluid

- Pressure is a cont. on a line if
  - line horizontal
  - stationary fluid is uniform and
  - stationary fluid is continuous.

### 3. Measurement of pressure

o pressure measurement devices:

- manometer for atmospheric pressure
- manometers for gage pressure using liquid columns.  
in vertical / inclined tubes.

— manometer & telegraphic pressure measuring devices.

- o in engineering normally use gage pressure.  
for abs pressure 'Pabs' must be indicated.

3.1 Mercury Barometer — find  $P_{atm}$

$$P_{atm} = P_{merg}$$

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3.2 Manometer tube — to find  $P_A$  & or  $P_1$

$$P_1 = P_{atm} + \gamma_1 h_1 \quad (\text{abs})$$

$$P_1 = \gamma_1 h_1 \quad (\text{gage})$$

3.3 U-Tube manometer —  $P_A$

$$\begin{aligned} P_2 &= P_3 \\ P_2 &= P_1 + \gamma_1 h_1 = P_A + \gamma_1 h_1 \\ P_3 &= P_{atm} + \gamma_2 h_2 \\ P_A &= P_{atm} + \gamma_2 h_2 - \gamma_1 h_1 \quad (\text{gage}) \end{aligned}$$

3.4 Differential U-tube manometer to find  $P_A - P_B$

$$\begin{aligned} P_A &= P_1 \\ P_1 + \gamma_1 h_1 &= P_2 \\ P_2 &= P_3 \\ P_3 - \gamma_2 h_2 &= P_4 \\ P_4 - \gamma_3 h_3 &= P_B \\ \text{Adding up: } P_A + \gamma_1 h_1 - \gamma_2 h_2 - \gamma_3 h_3 &= P_B \end{aligned}$$

Differential pressure:

$$P_A - P_B = \gamma_2 h_2 + \gamma_3 h_3 - \gamma_1 h_1$$

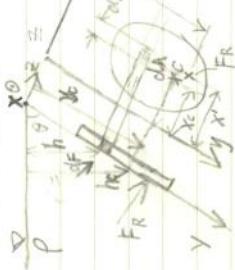
Inclined-tube manometer to find  $P_A - P_B$

inclined  
for better  
accuracy

$$\begin{aligned} P_A + \gamma_1 h_1 - \gamma_2 h_2 \sin \theta - \gamma_3 h_3 &= P_B \\ \text{OR: } P_A - P_B &= \gamma_2 h_2 \sin \theta + \gamma_3 h_3 - \gamma_1 h_1 \end{aligned}$$

4. Hydrostatic force on submerged surfaces
- o quantities of interests
    - magnitude of force
    - direction of force
    - location of action of force
  - o basic equation:
 
$$P = P_0 + \gamma h \quad \text{or} \quad P = \gamma y \quad \text{with} \quad y = Pg$$
  - o method - integration

- 4.1 Hydrostatic force on a flat surface
- o magnitude:  $F = P_c A$  ,  $P_c$  is pressure at centroid
  - o direction: perpendicular to surface
  - o location of action or centre of pressure - to find



$$F = P dA = \int_A P dy dA = \int_A \gamma y dA = \gamma \int_A y dA$$

$$\int_A y dA = y_c A \rightarrow \text{first moment of area}$$

~~from centroid of~~

$$F_R = P_c A = \gamma y_c A = \gamma A h = P_c b h \quad (\text{Force magnitude})$$

( $P_c$ : gauge pressure at centroid of A)

$F_{total} = \text{pressure at centroid} \times \text{total area}$ .

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Location of resultant force — pressure centre  
moment of resultant force about X-axis  
= moment due to distributed force about the same axis

$$y' F_R = \int_A y P dA = \int_A y \gamma y^2 \sin \theta dA$$

$$y' = \frac{\int_A y^3 \sin \theta dA}{F_R} = \frac{\int_A y \sin \theta A y^2 dA}{\int_A y \sin \theta dA} = \frac{\int_A y^2 dA}{y_c A} \quad \text{①}$$

$$\int_A y^2 dA = I_x \leftarrow \text{Second moment of the area about X-axis}$$

With parallel axis theorem:  
 $I_x = I_{xc} + A y_c^2 \quad \text{②}$

$I_{xc}$ : second moment of the area with respect  
to an axis passing through its centroid  
(and parallel to X-axis)

$$① + ② \Rightarrow y' = \frac{I_{xc}}{I_x} y + y_c \quad (\text{Force acting location})$$

↳ resultant does not pass through centroid as  
 $I_{xc}/(A) > 0$

$$\text{Similarly, } x' = \frac{I_{yc}}{y_c A} = x_c + \frac{I_{yc}}{y_c A}$$

where  $I_{xy} = \int_A xy dA = I_{xyc} + A x_c y_c$   
is product of inertia with respect to X & Y  
— direction of resultant force is perpendicular to the plane.

Exmps:

A: vertical wall, width b, height h

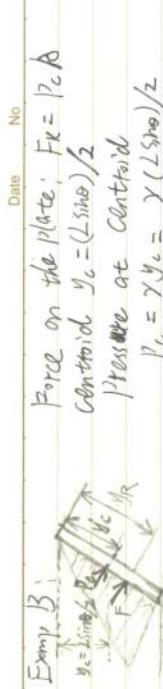
Force on plate

$$F_R = P_c A = \gamma y_c A = (\gamma h/2)(bh) = \gamma b h^2/2$$

$$P_c = \gamma h/2 \quad \text{pressure at centroid,}$$

pressure center:  $y_R = y_c + \frac{I_{xc}}{A y_c}$ , with  $I_{xc} = \frac{1}{12}bh^3$ ,  $\Rightarrow y_R = \frac{2h}{3}$

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$$\text{Area: } A = bL$$

$$F_R = \gamma (L \sin\theta) bL / 2 \times bL = (\gamma bL^2 \sin\theta) / 2$$

$$\text{Pressure center: } y_k = y_c + \frac{I_{xc}}{A y_c} \text{ with } y_k = L/2$$

$$\text{With: } I_{xc} = \frac{1}{12} bL^3, \quad A = bL$$

$$\Rightarrow \text{Pressure center: } y_k' = 2L/3$$

$$\text{Examp C: submerged vertical wall, with } b, \text{ height } h$$

$$\text{Force on plate: } F_R = p_c A$$

$$\text{Centroid: } y_c = \frac{(y_1 + y_2)}{2}$$

$$\text{Pressure at centroid: } p_c = \gamma y_c = \gamma (y_1 + y_2)/2$$

$$\text{Area: } A = bh$$

$$\text{Force on the plate: } F_R = bh\gamma (y_1 + y_2)/2$$

Location of action (Method 1):

$$y_k = y_c + \frac{I_{xc}}{A y_c} \text{ with } I_{xc} = \frac{bh^3}{12}, \quad A = bh$$

$$y_k = \frac{(y_1 + y_2)}{2} + \frac{bh^3}{bh} \times \frac{2}{y_1 + y_2} = \frac{(y_1 + y_2)}{2} + \frac{h^2}{6(y_1 + y_2)}$$

Method 2 for  $y_k$ : decompose pressure prism into two:  
 part1: rectangular,  $p_1$   
 part2: triangular,  $p_2 - p_1$

$$\text{Pressure Centers:}$$

$$\text{Part 1: } y_{k1} = y_1 + h/2$$

$$\text{Part 2: } y_{k2} = y_1 + 2h/3$$

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Using moment balance

$$F_R y_k = F_1 y_{k1} + F_2 y_{k2}$$

$$\text{where } F_1 = \gamma y_1 h b, \quad F_2 = \gamma h^2 b/2$$

Examp D: submerged by  $\theta$ , inclined wall, width  $b$ , length  $l$



$$F_R = p_c A$$

$$\text{Force on plate: } F_R = p_c A$$

$$\text{Centroid: } y_c = y_1 + (L \sin\theta)/2$$

$$\text{pressure at centroid: } p_c = \gamma y_c = \gamma [y_1 + (L \sin\theta)/2]$$

$$\text{Area: } A = bL$$

$$\text{Force on the plate: } F_R = p_c A = bL\gamma [y_1 + (L \sin\theta)/2]$$

pressure center (method 1):

$$y_k = y_1 + \frac{I_{xc}}{A y_c}, \quad \text{with } I_{xc} = \frac{1}{2} bL^3 \text{ and } A = bL$$

$$y_k = \frac{6y_1^2 + 6Ly_1 \sin\theta + 2L^2 \sin^2\theta}{3 \sin\theta (2y_1 + L \sin\theta)}$$

4.2 Force on a curved surface

$$F_R = p_c A$$

$$F_R = F_H + F_V$$

decompose force into  $F_H$  (vertical) and  $F_V$  (horizontal)

$F_H$ : Force on projected vertical plane surface  
 $F_V$ : Weight of liquid directly above  
 (Real: Imaginary:  $\uparrow$ )

$F_V$  passes through the center of the gravity of the liquid volume directly above the curved surface.

Resultant force:  $F_R = \sqrt{(F_H)^2 + (F_V)^2}$